Typical reaction on the fixings attaching the Constant Force post (CFp) to the roof structure


Constant Force post undeployed condition


Constant Force post deployed condition

## Reaction direction and nomenclature

Free body diagram of a deployed Constant Force post when attach directly onto a metal deck roofing system.


Tension force on fixings due to applied service load;

$$
F_{t}=\frac{S_{l} \times H}{L}
$$

And

$$
F_{t}=F_{c}
$$

Where;
$\mathrm{H}=$ Deployment height of the CFp $=35 \mathrm{~mm}\left(13 / \mathrm{s}^{\prime \prime}\right)$
$\mathrm{L}=$ Distance between the rear fixings and the leading edge of the baseplate (mm or in)
$S_{I}=$ Service load = Type 1 CFp 10 kN (2248lb), Type 2 CFp 6.5 kN (1461lb)
$\mathrm{F}_{\mathrm{t}}=$ Tension force on fixings (kN or lb)
$\mathrm{F}_{\mathrm{c}}=$ Compression force on fixings (kN or lb)
$F_{s}=$ Shear force on fixings (kN or lb)
$B_{n}=$ Quantity of fixings through the baseplate into the roof system

## CFp's using stitching screw/rivet, fixings

Calculated reactions for the stitching screw/rivets used with Type 1 and Type 2 CFp's installed directly onto a metal deck roofing system using 16 fixings (4 fixings at each corner of the baseplate).

Type 1 Constant Force Post reactions on fixings

| Fixing centres | H |  | L |  | $B_{n}$ | $S_{1}$ |  | $\mathrm{F}_{\mathrm{t}}$ |  | $F_{t}$ per fixing |  | Fs per fixing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | mm | in | mm | in |  | kN | lb | kN | lb | kN | lb | kN | lb |
| 300 | 35 | 1.38 | 312 | 12.3 | 16 | 10 | 2248 | 1.1 | 247.3 | 0.14 | 31.5 | 0.63 | 141 |
| 333 |  |  | 345 | 13.6 |  |  |  | 1.0 | 224.8 | 0.13 | 29.2 |  |  |
| 380 |  |  | 392 | 15.4 |  |  |  | 0.9 | 202.3 | 0.11 | 24.7 |  |  |
| 400 |  |  | 412 | 16.2 |  |  |  | 0.8 | 179.8 | 0.1 | 22.5 |  |  |
| 456 |  |  | 468 | 18.4 |  |  |  | 0.75 | 168.6 | 0.09 | 20.2 |  |  |
| 500 |  |  | 512 | 20.2 |  |  |  | 0.7 | 157.4 | 0.08 | 18.0 |  |  |

Type 2 Constant Force Post reactions on fixings

| Fixing centres | H |  | L |  | $B_{n}$ | SI |  | $\mathrm{F}_{\mathrm{t}}$ |  | $F_{t}$ per fixing |  | Fs per fixing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | mm | in | mm | in |  | kN | lb | kN | lb | kN | lb | kN | lb |
| 300 | 35 | 1.38 | 312 | 12.3 | 16 | 6.5 | 1461 | 0.73 | 164.1 | 0.09 | 22.5 | 0.41 | 92 |
| 333 |  |  | 345 | 13.6 |  |  |  | 0.66 | 148.4 | 0.08 | 18.0 |  |  |
| 380 |  |  | 392 | 15.4 |  |  |  | 0.58 | 130.4 | 0.073 | 16.4 |  |  |
| 400 |  |  | 412 | 16.2 |  |  |  | 0.55 | 123.6 | 0.07 | 15.7 |  |  |
| 456 |  |  | 468 | 18.4 |  |  |  | 0.5 | 112.4 | 0.06 | 13.5 |  |  |
| 500 |  |  | 512 | 20.2 |  |  |  | 0.45 | 101.2 | 0.055 | 12.4 |  |  |

## CFp's using clamps onto standing seam roofing

Calculated reactions for the load applied at the top of the clamp used when Type 1 and Type 2 CFp's are installed onto a standing seam roofing system using 4 clamps (1 clamp at each corner of the baseplate).

Where;
$\mathrm{H}=$ Deployment height of the CFp $=35 \mathrm{~mm}\left(13 / \mathrm{c}^{\prime \prime}\right)$
$\mathrm{L}=$ Distance between the centrelines of each clamp (mm or in)
$\mathrm{S}_{\mathrm{I}}=$ Service load = Type 1 CFp 10 kN (2248lb), Type 2 CFp 6.5 kN (1461lb)
$\mathrm{F}_{\mathrm{t}}=$ Tension force on clamps (kN or lb)
$\mathrm{F}_{\mathrm{c}}=$ Compression force on clamps (kN or lb)
$\mathrm{F}_{\mathrm{s}}=$ Shear force across clamps (kN or lb)
$B_{n}=$ Quantity of clamps fitted through the baseplate into the roof system

## Type 1 Constant Force Post reactions on clamps

| Standing seam | H |  | L |  | $B_{n}$ | SI |  | $\mathrm{F}_{\mathrm{t}}$ |  | $F_{t}$ per fixing |  | $\mathrm{F}_{\mathrm{s}}$ per fixing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| centres | mm | in | mm | in |  | kN | lb | kN | lb | kN | lb | kN | lb |
| 300 mm | 35 | 1.38 | 300 | 11.8 | 4 | 10 | 2248 | 1.17 | 263.0 | 0.58 | 130.4 | 2.5 | 562.0 |
| 333 mm |  |  | 333 | 13.1 |  |  |  | 1.05 | 236.0 | 0.53 | 119.1 |  |  |
| 400 mm |  |  | 400 | 15.7 |  |  |  | 0.88 | 197.8 | 0.44 | 98.9 |  |  |
| 500 mm |  |  | 500 | 19.7 |  |  |  | 0.7 | 157.4 | 0.35 | 78.7 |  |  |
| 537 mm |  |  | 537 | 21.1 |  |  |  | 0.65 | 146.1 | 0.33 | 74.2 |  |  |
| 600 mm |  |  | 600 | 23.6 |  |  |  | 0.58 | 130.4 | 0.29 | 65.2 |  |  |

Type 2 Constant Force Post reactions on clamps

| Standing seam centres | H |  | L |  | $B_{n}$ | $S_{1}$ |  | $\mathrm{F}_{\mathrm{t}}$ |  | $F_{t}$ per fixing |  | Fs per fixing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | mm | in | mm | in |  | kN | lb | kN | lb | kN | lb | kN | lb |
| 300 mm | 35 | 1.38 | 300 | 11.8 | 4 | 6.5 | 1461 | 0.76 | 170.9 | 0.38 | 85.4 | 1.6 | 365.3 |
| 333 mm |  |  | 333 | 13.1 |  |  |  | 0.68 | 152.9 | 0.34 | 76.4 |  |  |
| 400 mm |  |  | 400 | 15.7 |  |  |  | 0.57 | 128.1 | 0.28 | 62.9 |  |  |
| 429 mm |  |  | 429 | 16.9 |  |  |  | 0.53 | 119.1 | 0.27 | 60.7 |  |  |
| 434 mm |  |  | 434 | 17.1 |  |  |  | 0.52 | 116.9 | 0.26 | 58.5 |  |  |
| 444 mm |  |  | 444 | 17.5 |  |  |  | 0.51 | 114.7 | 0.25 | 56.2 |  |  |
| 500 mm |  |  | 500 | 19.7 |  |  |  | 0.45 | 101.2 | 0.23 | 51.7 |  |  |
| 537 mm |  |  | 537 | 21.1 |  |  |  | 0.42 | 94.4 | 0.21 | 47.2 |  |  |
| 600 mm |  |  | 600 | 23.6 |  |  |  | 0.38 | 85.4 | 0.19 | 42.7 |  |  |

The Safety Company

Reaction direction and nomenclature for warm flat roofing

Free body diagram of a deployed Constant Force post using toggle bolts, threaded expanded anchors and resin fixed anchors through insulation.


Tension force on fixings due to applied service load considering the thickness of insulation between the baseplate and the fixing into the structure;

$$
F_{t}=\frac{S_{l} \times\left(H+H_{1}\right)}{L}
$$

Where:
$F_{t}=$ Tension force on fixings (kN or lb)
$\mathrm{F}_{\mathrm{c}}=$ Compression force on fixings (kN or lb)
$\mathrm{F}_{\mathrm{s}}=$ Shear force on fixings (kN or lb)
$\mathrm{S}_{\mathrm{I}}=$ Service load = Type 1 CFp 10 kN (2248lb), Type 2 CFp 6.5 kN (1461lb)
$\mathrm{H}=$ Deployment height of the CFp $=35 \mathrm{~mm}\left(13 / \mathrm{s}^{\prime \prime}\right)$
$\mathrm{H}_{\mathrm{i}}=$ Insulation depth between the baseplate and the fixing surface
$L=$ Distance between the rear fixings and the leading edge of the baseplate (mm or in)

Considered loading on fixings when CFp is deployed in direction $A$ and $B$


## CFp's using toggle bolt fixings

Calculated reactions for the bolts used with CFp's installed through insulation onto metal, concrete or plywood deck roofing systems.

Type 1 Constant Force Post reactions on fixings

| Direction | H |  |  |  | Bn | L |  | SI |  | Fs per fixing |  | $F_{t}$ per fixing |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | mm | in | mm | in |  | mm | in | kN | lb | kN | lb | kN | lb |
| A | 35 | 1.4 | 50 | 2 | 4 | 293 | 8 | 10 | 2248 | 2.5 | 562 | 1.45 | 326 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 0.86 | 193 |
| A |  |  | 100 | 4 |  | 293 | 8 |  |  |  |  | 2.30 | 517 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 1.36 | 306 |
| A |  |  | 150 | 6 |  | 293 | 8 |  |  |  |  | 3.16 | 710 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 1.86 | 418 |
| A |  |  | 200 | 8 |  | 293 | 8 |  |  |  |  | 4.01 | 901 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 2.37 | 533 |
| A |  |  | 250 | 10 |  | 293 | 8 |  |  |  |  | 4.86 | 1092 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 2.87 | 645 |

Type 2 Constant Force Post reactions on fixings

| Direction |  |  |  |  | $B_{n}$ |  |  |  |  |  |  |  | ing |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | mm | in | mm | in |  | mm | in | kN | lb | kN | lb | kN | lb |
| A | 35 | 1.4 | 50 | 2 | 4 | 293 | 8 | 6.5 | 1461 | 1.6 | 366 | 0.94 | 211 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 0.56 | 126 |
| A |  |  | 100 | 4 |  | 293 | 8 |  |  |  |  | 1.50 | 337 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 0.88 | 198 |
| A |  |  | 150 | 6 |  | 293 | 8 |  |  |  |  | 2.05 | 461 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 1.21 | 272 |
| A |  |  | 200 | 8 |  | 293 | 8 |  |  |  |  | 2.61 | 587 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 1.54 | 346 |
| A |  |  | 250 | 10 |  | 293 | 8 |  |  |  |  | 3.16 | 710 |
| B |  |  |  |  |  | 496 | 18 |  |  |  |  | 1.87 | 420 |

Note: For combined reactions consult a suitably qualified engineer

| Reference | Date issued | Revision | Engineer |
| :---: | :---: | :---: | :---: |
| RFEA4070 | Jan 2022 | 04 | AW |

